

10000 (30' D-F 8000 DE. 7000 6000 (20) (II) 5000 ⟨0 4000 3000 Maximum allowable soil bearing capacity for walls founded in 2000 embankment is 400 kPa (4 TSE) 1000 800 (3) (4) (7) Foundation pressure in kPa (TSF) VERTICAL WALL

### DESIGN EXAMPLES

#### EXAMPLE NO.1

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Given: Wall height 7300 millimeters (24').

Design loading Case II.

35 meters (U5) maximum from toe of wall to top of slope.

Base in embankment 1500 millimeters (5') depth minimum.

Select: 6:1(1:6) Battered wall. Vertical wall not permitted. Type D wall selected.

Maximum height on graph is 8050 millimeters (26') at 400 kPa (4 TSF). Since
the foundation pressure is 305 kPa (3 TSF) at 7300 millimeters (24'), the wall must
be founded on a 1500 millimeters (5') thickness of "Structure Backfill"

be founded on a I500 millimeters (5') thickness of "Structure Backfill" (See Design Note I). A drainage system behind this wall will be required.

EXAMPLE NO. 2

Given: Wall height 8800 millimeters (29').

Design loading Case II . Foundation site investigation indicates lateral pressure from material above will be equivalent to 1:2 (2:1) embankment slope. Base in excavation level at toe of wall. Foundation investigation determines the allowable soil bearing capacity at 300 KPa (3 TSF).

Select: Battered Type F wall maximum height at 400 kPa (4 TSF) is 9000 millimeters (29°), therefore the replacement of 1500 millimeters (5°) of excavation with "Structure Backfill" to increase the allowable soil bearing capacity to 400 kPa (4 TSF) is required (See Design Note 1). A drainage system for this wall should be investigated.

EXAMPLE NO. 3

Given: Wall height 4600 millimeters (15').

Design loading Case III with overall height less than 35 meters (115"). Base on original ground. Foundation investigation determines allowable soil bearing appacity at 200 kPa (2 TSF).

Select: Use a Type C Battered Wall. However foundation pressure exceeds 200 kPa (2 TSF), therefore the replacement of 1500 millilmeters (5') of excavation with "Structure Backfill" to increase the allowable soil bearing capacity to 400 kPa (4 TSF) is required (See Desian Note I).

#### LEGEND FOR GRAPHS

A, B, C, D, E, F = Wall Type  $(\widehat{I}).(\widehat{II}).(\widehat{II}) = \text{Loading Case}$ 

- For description of loading case, see detail of design loading cases.
- Upper end of line indicates maximum wall height for a given wall type.

# II.5 kPa (0.12 TSF) Surcharge $\sim$

# TOWN (SEI) WWW 000 SE CASE III CASE III CASE III

## DETAIL OF DESIGN LOADING CASES

Note: Similar loading cases for vertical wall.

#### NOTE

For construction details see Sheet C8A.

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#### DESIGN NOTES

- I. Wall base in embankment: A minimum depth of ISO0 mm (5') of embankment at 95% relative soil compaction is required below the base of all walls in order to constitute an embankment condition. When the foundation pressure is between 240 kPa and 400 kPa (2.5 TSF and 4 TSF), embankment below the wall shall consist of "Structure Backfill" material as set forth in Section 19-3.06 of the Standard Specifications. The limits of relative compaction (95 percent) shall be as set forth in Section 19-5.03 of the Standard Specifications.
- 2. Wall base in original ground: Allowable soil pressure at toe of wall shall be determined by foundation site investigation. Walls that are to retain out slopes shall be designed for lateral and toe pressures determined from site investigation data. Overall stability of slope with wall in place must be analyzed. If original ground slopes away from toe of wall, reduction in allowable bearing capacity due to slope must be considered. Walls shall not be founded in original ground having an allowable bearing capacity of less than 145 kPa. Consideration should be given to removal and replacement of unsuitable material with "Structure Backfill" material as set forth in Section 19-3.06 of the Standard Specifications. The limits of relative compaction (95 percent) shall be as set forth in Section 19-5.03 of the Standard Specifications.

#### 3. Drainage

- a. Internal: Section 19-3.06 of the Standard Specifications.
- b. External: If the combined height of wall and overfill (measured along face of wall and vertically from the toe of fill to top of fill exceeds 7600 mm (25) a system to drain water away from the back face of wall shall be provided. The type and extent of this system will depend on the type of backfill material expected to be used, the combined height of wall and backfill, and the location of the water table in the area.
- 4. Sloping Surcharge Limitations: The maximum height of fill behind any wall, or family of walls, shall not exceed 35 meters (115') (measured vertically from the toe of the bottom wall to the top of fill behind the uppermost wall). For a family of walls the slope of a line drawn from the top of the front face of the bottom wall to the top of the front face of any intermediate or top wall, shall in no case be steeper than 1:15 (11/2:1).

#### Material Specifications:

Steel sheets:
AASHTO M2I8
310 275 kPa (45,000 psi) Ultimate
227 535 kPa (35,000 psi) Yield
a 20% Elongation
Boltss ASTM A307 Grade A

STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION

# STEEL CRIB WALL DESIGN DATA

These "Standard Plans for Construction of Local Streets and Roads" contain units in two systems of measurement: International System of Units (SI or "metric") and United States Standard Measures shown in the parentheses (). The measurements expressed in the two systems are not necessarily equal or interchangeable. See the "Foreword" at the beginning of this publication.

NO SCALE